



The Influence of GPL Reinforcements on the Post-Buckling Behavior of FG Porous Rings Subjected to an External Pressure

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Abstract: The work focuses on the post-buckling behavior of functionally graded graphene platelet (FG-GPL)-reinforced porous thick rings with open-cell internal cavities under a uniform external pressure. The generalized rule of mixture and the modified Halpin–Tsai model are here used to evaluate the effective mechanical properties of the ring. Three types of porosity patterns are assumed together with five different GPL distributions as reinforcement across the ring thickness. The theoretical formulation relies on a 2D-plane stress linear elasticity theory and Green strain field in conjunction a virtual work principle to derive the nonlinear governing equations of the post-buckling problem. Unlike the simple ring models, 2D elasticity considers the thickness stretching. The finite element model combined with an iterative Newton–Raphson algorithm is used to obtain the post-buckling path of the ring up to the collapse. A systematic investigation evaluates the effect of the weight fraction of nanofillers, the coefficient of porosity, porosity distribution, and the GPLs distribution on the deep post-buckling path of the ring. Based on the results, it is found that the buckling value and post-buckling strength increase considerably (by approximately 80%) by increasing the weight fraction of the nanofiller of about 1%.

Keywords: ring; post-buckling-functionally graded; porous; graphene platelets; uniform pressure**MSC:** 37M20

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1. Introduction

Rings are considered as a major solid structure largely used as reinforcement for closed shell-like constructions and offshore pipelines. In many applications, rings are usually subjected to a compression loading undergoing buckling phenomena. An adequate knowledge of the post-buckling behavior will be useful to designers to calculate the remaining capacity of such structures. Many investigations have been reported in the literature about the buckling and post-buckling behavior of rings. For instance, Wah [1] presented a simple model for evaluating the in-plane buckling of rings subjected to uniform compressive load. Hazel and Mullin [2] performed a double computational and experimental investigation on the elastic buckling response of rings under an external confinement, while using a Kirchhoff–Love beam theory. Azzuni and Guzey [3] presented an analytical solution based on a perturbation approach for the post-buckling analysis of rings under non-uniform double symmetric loads. The elastic buckling of rings subjected to a uniform external pressure was also analyzed by Seide and Weingarten [4]. Azzuni and Guzey [5] applied an analytical study for the deflection and buckling behavior of rings with large deflections. Kerdegarbakhsh et al. [6] employed the first order shear deformation theory (FSDT) to analyse the post-buckling behavior of FGM rings, whose solution was found numerically based on a generalized differential quadrature method (GDQM). Lagrange et al. [7] investigated analytically and numerically the buckling response of a ring resting on an

annular elastic foundation. Thurston [8] also studied the impact of the loading type on the post-buckling response of a ring. Thurston employed the general Newton procedure to derive the buckling pressures and post-buckling paths of the closed rings. Sills and Budiansky [9] stated that differently from rings under pure hydrostatic, constant radial, and directional forces, rings are commonly exposed to inverse-square radial loads with a nonlinear bending behavior. Asemi and Kiani [10] employed a 2D elasticity theory to investigate the post-buckling behavior of orthotropic rings by means of the finite element approach. Zhang et al. [11] applied a classical shell theory and proposed an analytical solution for the stability problem of submarine bi-material pipeline-liner systems constructed of novel polyhedral composites under a thermo-mechanical loading.

At the same time, porous cellular constructions with great properties, including lightweight, heat resistance, and great energy absorption, have been extensively applied in different systems [12–16]. In addition, Mousavi et al. [17] proposed an analytical formulation based on a trigonometric theory to describe the initial curvature influences on the free vibration response of porous micro beams combined with FG piezoelectric layers. Bouhadra, Menasria, and Rachedi [18] investigated the effect edge conditions for the buckling behavior of porous FG nanobeams based on a quasi-2D and 3D formulation. Keleshteri and Jelovica [19] studied the nonlinear vibration analysis of bidirectional porous beams based on a Reddy beam theory, while employing the GDQM. A refined third-order theory was also proposed by Beg et al. [20] for the study of the static and free vibration response of functionally graded porous curved beams. Akbaş [21] investigated the dynamics of axially FG porous beams subjected to a moving force, as provided by the FSDT beam theory and Ritz procedure. Huang and Tahouneh [22] performed an investigation about the natural frequencies of FG porous material beam on a two-parameter elastic medium according to a Timoshenko theory and differential quadrature approach. Madenci and Ozkili [23] performed a comprehensive investigation, including the analytical and numerical approaches for natural frequencies of open-cell FG porous beams. Rahmani and Mohammadi [24] studied the vibration behavior of FG porous conical shells for different boundary conditions by employing a high-order shell theory and Galerkin method to solve the problem. The wave propagation of porous FG curved beams in thermal conditions was also presented by Xu et al. [25], whereas Farrokh and Taheripur [26] studied the optimization topology of FG porous beams under a buckling load based on the Euler and Timoshenko beam theories. Yuksel and Akbas [27] presented an analytical solution for the hygrothermal stress analysis of laminated composite porous plates based on the FSDT. Moreover, Chen et al. [28] proposed a nonlocal strain gradient finite element method to assess the nonlinear response of FGM porous microplates with a central cutout. The natural frequencies of porous magneto-electro-elastically actuated CNT-reinforced composite sandwich plates was presented by Ebrahimi et al. [29] by employing a refined plate theory.

However, Xia et al. [30] and Jena and Malikan [31] stated that the structural stiffness decreases considerably due to the internal cavities in the matrix. To overcome this drawback, different nanofillers, e.g., CNTs or GPLs, were immersed in the matrix phase [32–36] as a useful method to enhance their properties. To enhance the potential properties of the structures, FG porous constructions reinforced by GPLs were introduced by Hassani et al. [37] and Dong et al. [38]. Different investigations have been conducted to study the effect of porosity and GPLs distributions on these structures.

In the following, we report some further key works related to the buckling and post-buckling response of such structures. More specifically, a FSDT and Rayleigh–Ritz method was applied in Refs. [39,40], to analyze the buckling [39] and torsional buckling [40] of FG-GPL porous cylindrical shells. Dong et al. [41] also employed a FSDT for an analytical investigation of the buckling response of FG-GPL porous cylindrical shells. A further analytical treatment of the nonlinear vibration and post-buckling problems based on a Ritz method was proposed by Chen et al. [42] focusing on FG-GPLs porous nanocomposite beams as useful for design purposes of many engineering nanodevices. Kitipornchai et al. [43] investigated the free vibration and buckling responses of FG porous beams

reinforced by GPLs. Similarly, Yang et al. [44] studied the buckling load of FG-GPL porous plates based on the FSDT, while proposing the Chebyshev–Ritz method as a valid analytical solution. A classical shell theory and higher-order shear deformation theory (HSDT) were used by Zhou et al. [45] to determine the nonlinear buckling response of FG-GPL porous cylindrical shells, whereas the same method was implemented in [46] for the buckling and post-buckling analyses of FG-GPL porous plates with different shapes and edge conditions. In addition, a closed-form solution was proposed by Yaghoobi and Taheri [47] to investigate the buckling behavior of sandwich plates with a FG-GPL porous core, with an extension of the solution in [48] to the post-buckling response of sandwich cylindrical shell panels with a FG-GPL porous core. An advanced isogeometric approach was, instead, proposed by Nguyen et al. [49] to treat an affine buckling problem of FG-GPL porous plates. Priyanka et al. [50] considered the effect of different porosities and GPL patterns on the natural frequencies and buckling forces of FG-GPL porous cylindrical panels always resorting to the HSDT. Barati and Zenkour [51] applied a Euler theory to capture the post-buckling paths of FG-GPL porous beams by employing a Galerkin solution. Li et al. [52] evaluated the buckling performance of FG porous composite pipelines with various shapes reinforced by GPLs under an external pressure. Similarly, Xiao et al. [53] proposed an analytical model for the nonlinear buckling response of a confined polyhedral FG porous-GPLs lining subjected to a point loading, whereas in Ref. [54], the same authors applied the nonlinear thin-walled shell theory and principle of minimum potential energy to investigate the nonlinear in-plane instability of confined FGP arches with nanocomposites reinforcements under a radial uniform pressure.

According to the most important literature overview mentioned above, it is worth noting that the post-buckling of rings constructed of FG porous material reinforced by GPLs has not been surveyed, so far. Due to the common presence of buckling loading conditions in most ring structures in many industries, large attention must be devoted by researchers and designers to the knowledge of critical conditions and structural capacity, especially in post-buckling situations. In such a context, this work focuses on the post-buckling response of FG porous rings reinforced by GPLs according to a 2D elasticity theory in lieu of simple shell theories. The proposed theory considers the thickness stretching effects and predicts the post-buckling behavior accurately, involving the nonlinear Green strain field rather than a Von Kármán nonlinearity. An FEM-based approach is combined to a Newton–Raphson iterative scheme to derive the post-buckling paths of FG-GPL porous rings. A systematic study is here performed to evaluate the effect of various porosity distributions and GPL patterns, porosity coefficients, and weight fractions of the nanofiller on the post-buckling response of FG porous thick closed rings, whose results could serve for design purposes of such structural components.

2. Theoretical Formulation

2.1. Description of the Geometry

Let us consider a closed circular ring in polar coordinates, as shown in Figure 1, in which h , R , r_{out} , and r_{in} are thickness, mean radius, and outer and inner radii of the ring, respectively.

2.2. Obtaining the Effective Mechanical Properties of the Ring

Three various porosity patterns are considered across the ring thickness (Figure 2), as described hereafter, together with five GPL dispersion patterns across the thickness of ring [55–57]. More follows in detail hereafter, and we will describe the elasticity modulus $E(r)$, shear modulus $G(r)$, and mass density $\rho(r)$ of porous nanocomposite ring according to Ref. [58].

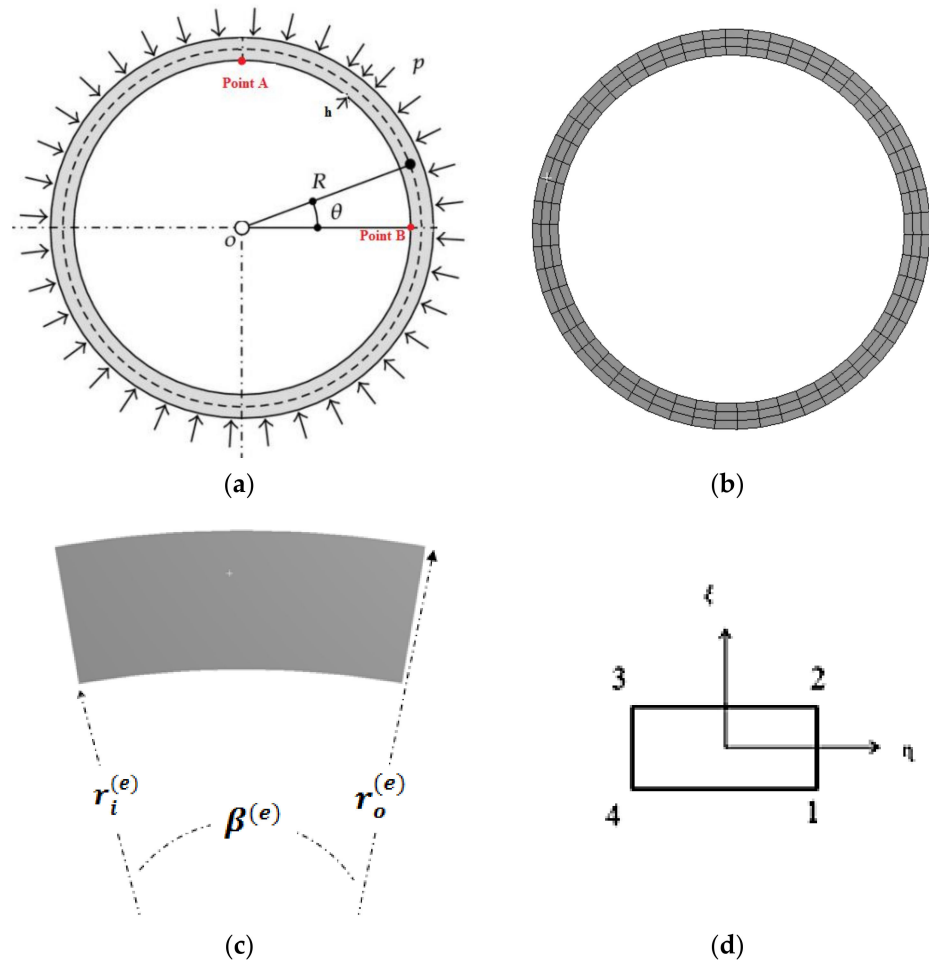


Figure 1. Geometry and coordinate system of ring (a), meshed ring (b), annular sector element (c), and local coordinate system (d).

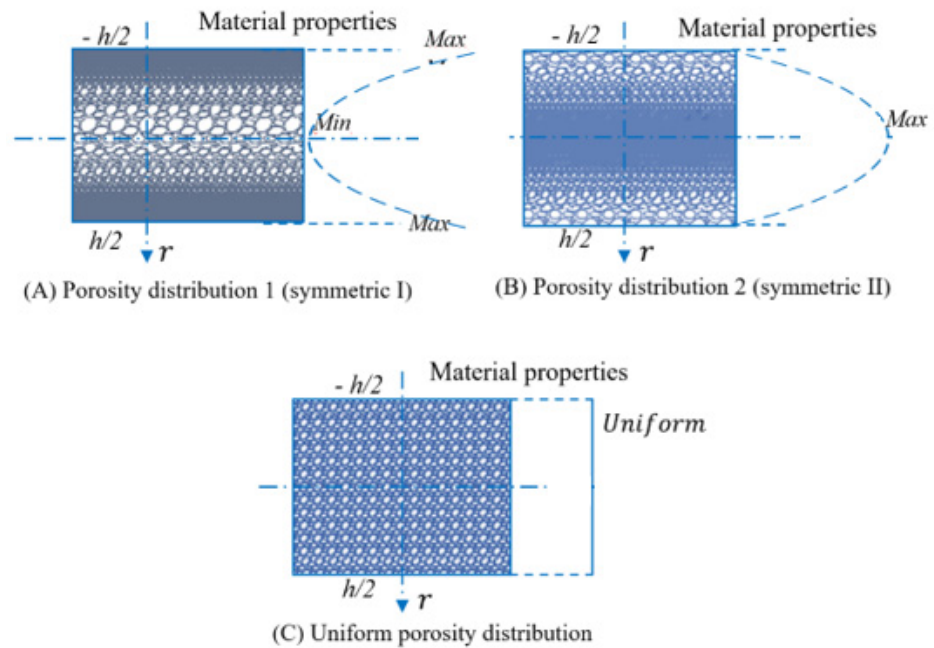


Figure 2. Cont.

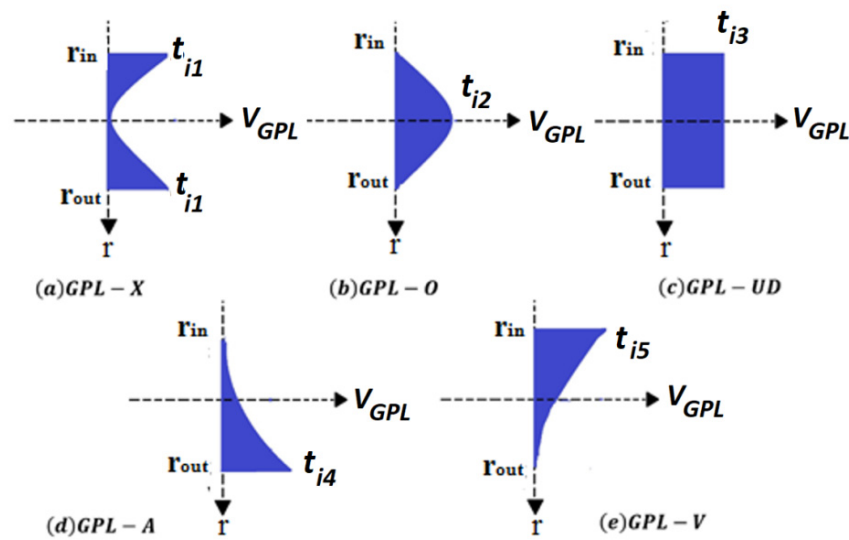


Figure 2. Porosity patterns and GPL distributions.

Porosity distribution 1 (non-uniform symmetric I-PD1):

$$\begin{cases} E(r) = E^* \left[1 - e_0 \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right] \\ G(r) = G^* \left[1 - e_0 \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right] \\ \rho(r) = \rho^* \left[1 - e_m \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right] \end{cases} \quad (1)$$

Porosity distribution 2 (non-uniform symmetric II-PD2):

$$\begin{cases} E(r) = E^* \left[\left(1 - e_0^* \left(1 - \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right) \right) \right] \\ G(r) = G^* \left[\left(1 - e_0^* \left(1 - \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right) \right) \right] \\ \rho(r) = \rho^* \left[\left(1 - e_m^* \left(1 - \cos\left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2}\right)\right) \right) \right) \right] \end{cases} \quad (2)$$

Porosity distribution 3 (PD3):

$$\begin{aligned} E &= E^* \alpha \\ G &= G^* \alpha \\ \rho &= \rho^* \alpha' \end{aligned} \quad (3)$$

where E^* , G^* , and ρ^* refer to the elasticity modulus, shear modulus, and mass density of the GPL ring without internal pores, respectively. Also, e_0 and e_0^* ($0 \leq e_0(e_0^*) < 1$) represent the porosity coefficients for pattern 1 and 2, respectively. Note also that the porosity coefficient for the third distribution is labeled as α , whereas e_m and e_m^* refer to the mass density coefficient for distributions 1 and 2, respectively. Also, the mass density coefficient for distribution 3 is presented by the α' symbol.

The Young's modulus and density for open-cell metal foams are related as [59–61].

$$\frac{E(r)}{E^*} = \left(\frac{\rho(r)}{\rho^*} \right)^2 \quad (4)$$

whose equation is applied to estimate the relation between the porosity coefficients and mass density coefficients for different porosity distributions, as follows:

$$\begin{aligned}
 1 - e_m \cos(\pi r) &= \sqrt{1 - e_0 \cos(\pi r)} \\
 1 - e_m^* \cos(\pi r) &= \sqrt{1 - e_0^* \cos(\pi r)} \\
 \alpha' &= \sqrt{\alpha}
 \end{aligned}
 \tag{5}$$

It is supposed that the mass of various rings with different distributions of porosity and GPL dispersion patterns are the same.

$$\int_{rin}^{rout} \sqrt{1 - e_0 \cos(\pi r)} dr = \int_{rin}^{rout} \sqrt{1 - e_0^* (1 - \cos(\pi r))} dr = \int_0^{h/2} \sqrt{\alpha} dr
 \tag{6}$$

Based on Equation (6), the amounts of e_0^* and α may be evaluated with a known value of e_0 , as given in Table 1. By increasing e_0 , the amounts of e_0^* increase while α decrease. When e_0 reaches 0.6, e_0^* becomes equal to 0.9612. Thus, $e_0 \in [0, 0.6]$ is used in this work.

Table 1. Porosity coefficients for PD1, PD2, and PD3.

e_0	e_0^*	α
0.1	0.1738	0.9361
0.2	0.3442	0.8716
0.3	0.5103	0.8064
0.4	0.6708	0.7404
0.5	0.8231	0.6733
0.6	0.9612	0.6047

The Young’s modulus of the nanocomposite without any porosity is here computed based on the Halpin–Tsai micromechanics model [62–64], as follows:

$$E^* = \frac{3}{8} \left(\frac{1 + \epsilon_L^{GPL} \eta_L^{GPL} V_{GPL}}{1 - \eta_L^{GPL} V_{GPL}} \right) E_m + \frac{5}{8} \left(\frac{1 + \epsilon_W^{GPL} \eta_W^{GPL} V_{GPL}}{1 - \eta_W^{GPL} V_{GPL}} \right)
 \tag{7}$$

in which

$$\epsilon_L^{GPL} = \frac{2l_{GPL}}{t_{GPL}}
 \tag{8}$$

$$\epsilon_w^{GPL} = \frac{2w_{GPL}}{t_{GPL}}
 \tag{9}$$

$$\eta_L^{GPL} = \frac{E_{GPL} - E_m}{E_{GPL} + \epsilon_L^{GPL} E_m}
 \tag{10}$$

$$\eta_W^{GPL} = \frac{E_{GPL} - E_m}{E_{GPL} + \epsilon_W^{GPL} E_m}
 \tag{11}$$

Note that subscripts GPL and m refer to the mechanical properties of GPL and matrix of metallic nanocomposite, respectively. The volume content of GPLs is shown with V_{GPL} , whereas symbols l_{GPL} , w_{GPL} , and t_{GPL} are used for indicating the length, width, and thickness of nanofiller platelets, respectively.

Applying the rule of mixture, the mass density and Poisson’s ratio of the nanocomposite are evaluated as follows [65–67]:

$$\rho^* = \rho_{GPL}V_{GPL} + \rho_m(1 - V_{GPL}) \tag{12}$$

$$v^* = v_{GPL}V_{GPL} + v_m(1 - V_{GPL}) \tag{13}$$

The shear modulus G^* of the nanocomposite structure is defined as:

$$G^* = \frac{E^*}{2(1 + v^*)} \tag{14}$$

It is mentioned that the volume content of GPLs, V_{GPL} , for various GPL dispersion patterns varies across the ring thickness and can be estimated by the following relation (see also Figure 2):

$$V_{GPL}(z) = \left\{ \begin{array}{ll} t_{i1} \left[1 - \cos \left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2} \right) \right) \right] & GPL - X \\ t_{i2} \cos \left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2} \right) \right) & GPL - O \\ t_{i3} & GPL - UD \\ t_{i4} \left[1 - \cos \left(\pi \left(\frac{r-r_{in}}{2h} \right) \right) \right] & GPL - A \\ t_{i5} \cos \left(\pi \left(\frac{r-r_{in}}{2h} \right) \right) & GPL - V \end{array} \right\} \tag{15}$$

where $t_{i1}, t_{i2}, t_{i3}, t_{i4}$, and t_{i5} show the upper limit of the V_{GPL} , and subscript $i = 1, 2$, and 3 show different porosity patterns, 1, 2, and 3, within each distribution. V_{GPL}^T is the total volume content of GPL and it is evaluated by employing the nanofiller weight fraction γ_{GPL} into Equation (16), then it is employed to obtain $t_{i1}, t_{i2}, t_{i3}, t_{i4}$, and t_{i5} by Equation (17).

$$V_{GPL}^T = \frac{\gamma_{GPL}\rho_m}{\gamma_{GPL}\rho_m + \rho_{GPL} - \gamma_{GPL}\rho_{GPL}} \tag{16}$$

$$V_{GPL}^T \int_{r_{in}}^{r_{out}} \frac{\rho(r)}{\rho^*} dr = \left\{ \begin{array}{l} t_{i1} \int_{r_{in}}^{r_{out}} \left[1 - \cos \left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2} \right) \right) \right] \frac{\rho(r)}{\rho^*} dr \\ t_{i2} \int_{r_{in}}^{r_{out}} \cos \left(\pi \left(\frac{r-r_{in}}{h} - \frac{1}{2} \right) \right) \frac{\rho(r)}{\rho^*} dr \\ t_{i3} \int_{r_{in}}^{r_{out}} \frac{\rho(r)}{\rho^*} dr \\ t_{i4} \int_{r_{in}}^{r_{out}} \left[1 - \cos \left(\pi \left(\frac{r-r_{in}}{2h} \right) \right) \right] \frac{\rho(r)}{\rho^*} dr \\ t_{i5} \int_{r_{in}}^{r_{out}} \cos \left(\pi \left(\frac{r-r_{in}}{2h} \right) \right) \frac{\rho(r)}{\rho^*} dr \end{array} \right. \tag{17}$$

2.3. Stress–Strain Relation

The constitutive relations of the elastic ring under the 2D elasticity plane stress assumptions are defined as

$$\sigma = D \varepsilon \tag{18}$$

where the stress σ and strain components ε are defined as

$$\sigma = \begin{Bmatrix} \sigma_{rr} \\ \sigma_{\theta\theta} \\ \sigma_{r\theta} \end{Bmatrix}, \varepsilon = \begin{Bmatrix} \varepsilon_{rr} \\ \varepsilon_{\theta\theta} \\ \gamma_{r\theta} \end{Bmatrix}. \tag{19}$$

and the elastic matrix D is defined as

$$D = \begin{bmatrix} \frac{E^*}{1-\nu^{*2}} & \frac{E^*}{1-\nu^{*2}} & 0 \\ \frac{\nu^* E^*}{1-\nu^{*2}} & \frac{E^*}{1-\nu^{*2}} & 0 \\ 0 & 0 & G^* \end{bmatrix}. \tag{20}$$

The Green strain components in polar coordinates are involved in the following relation

$$\varepsilon = \varepsilon_L + \varepsilon_{NL} \tag{21}$$

where ε_L is the linear strains, defined as

$$\varepsilon_L = \begin{Bmatrix} u_{,r} \\ \frac{1}{r}(u + u_{,\theta}) \\ \frac{1}{r}(u_{,\theta} - u + ru_{,r}) \end{Bmatrix} \tag{22}$$

and ε_{NL} is the nonlinear Green strain field, defined as

$$\varepsilon_{NL} = \frac{1}{2} \begin{Bmatrix} u_{,r}^2 + u_{,\theta}^2 \\ \frac{1}{r^2}(u_{,\theta} - v)^2 + \frac{1}{r^2}(v_{,\theta} + u)^2 \\ \frac{2}{r}(u_{,r}u_{,\theta} - vu_{,r} + v_{,r}v_{,\theta} + uv_{,r}) \end{Bmatrix} \tag{23}$$

where u and v stand for the radial and circumferential displacements, respectively.

3. Finite Element Modelling

The ring is segmented with 2D annular sector elements, where $r_i^{(e)}$, $r_o^{(e)}$, and $\beta^{(e)}$ are the inner radii, outer radii, and sector angle of each element, respectively [68,69].

The relations between polar and natural coordinates for each element are expressed as the following

$$\zeta = \frac{2r - r_o^{(e)} - r_i^{(e)}}{r_o^{(e)} - r_i^{(e)}}, \eta = \frac{2(\theta - \theta_c)}{\beta^{(e)}}, \tag{24}$$

where ζ and η are the natural coordinates along the r and θ axes, respectively, and θ_c is the global coordinate of the center for each element.

The matrix form of the linear strain field (22) is defined as

$$\varepsilon_L = \Gamma Q \tag{25}$$

with the matrix operator Γ defined as

$$\Gamma = \begin{bmatrix} \frac{\partial}{\partial \xi} - \frac{2}{r^{(e)}_o - r^{(e)}_i} & 0 \\ \frac{2}{\xi(r^{(e)}_o - r^{(e)}_i) + r^{(e)}_i - r^{(e)}_o} & \frac{4}{\xi(r^{(e)}_o - r^{(e)}_i) + r^{(e)}_i - r^{(e)}_o} \frac{\partial}{\beta^{(e)} \partial \eta} \\ \frac{4}{\xi(r^{(e)}_o - r^{(e)}_i) + r^{(e)}_i - r^{(e)}_o} \frac{\partial}{\beta^{(e)} \partial \eta} & \frac{\partial}{\partial \xi} \frac{2}{r^{(e)}_o + r^{(e)}_i} - \frac{2}{\xi(r^{(e)}_o - r^{(e)}_i) + r^{(e)}_i - r^{(e)}_o} \end{bmatrix} \tag{26}$$

which is applied to the following displacement vector Q

$$Q = \begin{Bmatrix} u \\ v \end{Bmatrix} \tag{27}$$

A 2D FEM is applied to approximate the displacement field. A 2D four-noded element is employed for segmenting the solution domain. The displacement vector Q may be related to the nodal displacement vectors of the element $\delta^{(e)}$ by the following relation

$$Q = N\delta^{(e)} \tag{28}$$

where the displacement field vector of for each element is defined as

$$\delta^{(e)} = \{U_1, V_1, U_2, V_2, U_3, V_3, U_4, V_4\}^T \tag{29}$$

The shape functions matrix takes the following form

$$N = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 & N_4 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 & 0 & N_4 \end{bmatrix} \tag{30}$$

The shape functions according to the natural coordinates are assumed as

$$N_i(\xi, \eta) = \frac{1}{4}(1 + \xi_i \xi)(1 + \eta_i \eta), \tag{31}$$

where ξ_i is the amount of the natural coordinate ξ for the i th node, e.g., if the i th node has coordinates $(1, -1)$, it is $\xi_i = 1$ and $\eta_i = -1$.

The virtual work principle is now employed to derive the nonlinear governing equations of an element

$$\delta \Pi = \delta U - \delta W = 0, \tag{32}$$

where U and W refer to the strain energy and external work associated to the in-plane external pressure q , respectively:

$$\begin{aligned} \delta U &= \int_A \delta \varepsilon^T \sigma dA, \\ \delta W &= \int_A^{2\pi} q \delta u|_{r=r_{out}} r_{out} d\theta. \end{aligned} \tag{33}$$

The Green strains and its variations in matrix form read as follows

$$\begin{aligned} \varepsilon &= B\delta^{(e)}, \\ \delta \varepsilon &= \bar{B}\delta\delta^{(e)}, \end{aligned} \tag{34}$$

where $B = B_L + B_{NL}$ and $\bar{B} = B_L + \bar{B}_{NL}$ refer to the partial differential derivative matrix of Green strains and its variational version, respectively, as derived by Equation (22). According to the global and natural coordinates, this matrix can be presented as

$$B_L = [B_L^{(1)} B_L^{(2)} B_L^{(3)} B_L^{(4)}] \tag{35}$$

and each submatrix $B_L^{(i)}$, $i = 1, 2, 3, 4$ will be as

$$B_L^{(i)} = \begin{bmatrix} \frac{\partial N_i}{\partial r} & 0 \\ \frac{N_i}{r} & \frac{1}{r} \frac{\partial N_i}{\partial \theta} \\ \frac{1}{r} \frac{\partial N_i}{\partial \theta} & \frac{\partial N_i}{\partial r} - \frac{N_i}{r} \end{bmatrix} \tag{36}$$

Moreover, the submatrix $B_L^{(i)}$ in natural coordinates is as

$$B_L^{(i)} = \begin{bmatrix} \frac{\zeta_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} & 0 \\ \frac{(1+\zeta_i\zeta)(1+\eta_i\eta)}{2(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} & \frac{\eta_i(1+\zeta_i\zeta)}{\beta^{(e)}(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \\ \frac{\eta_i(1+\zeta_i\zeta)}{\beta^{(e)}(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} & \frac{\zeta_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} - \frac{(1+\zeta_i\zeta)(1+\eta_i\eta)}{2(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \end{bmatrix} \tag{37}$$

whereas

$$B_{NL} = [B_{NL}^{(1)} B_{NL}^{(2)} B_{NL}^{(3)} B_{NL}^{(4)}], \tag{38}$$

whose components for each submatrix $B_{NL}^{(i)}$, $i = 1, 2, 3, 4$ will be defined as

$$\begin{aligned} B_{NL}^{(i)(1,1)} &= \frac{1}{2} \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{\partial N_j}{\partial r} U_j, \\ B_{NL}^{(i)(1,2)} &= \frac{1}{2} \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{\partial N_j}{\partial r} V_j, \\ B_{NL}^{(i)(2,1)} &= \frac{1}{2} \frac{\partial N_i}{r \partial \theta} \sum_{j=1}^4 \frac{\partial N_j}{r \partial \theta} U_j - \frac{\partial N_i}{r \partial \theta} \sum_{j=1}^4 \frac{N_j}{r} V_j + \frac{1}{2} \frac{N_i}{r} \sum_{j=1}^4 \frac{N_j}{r} U_j, \\ B_{NL}^{(i)(2,2)} &= \frac{1}{2} \frac{\partial N_i}{r \partial \theta} \sum_{j=1}^4 \frac{\partial N_j}{r \partial \theta} V_j - \frac{\partial N_i}{r \partial \theta} \sum_{j=1}^4 \frac{N_j}{r} U_j + \frac{1}{2} \frac{N_i}{r} \sum_{j=1}^4 \frac{N_j}{r} V_j, \\ B_{NL}^{(i)(3,1)} &= \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{\partial N_j}{r \partial \theta} U_j - \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{N_j}{r} V_j, \\ B_{NL}^{(i)(3,2)} &= \frac{1}{2} \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{\partial N_j}{r \partial \theta} V_j - \frac{\partial N_i}{\partial r} \sum_{j=1}^4 \frac{N_j}{r} U_j. \end{aligned} \tag{39}$$

The submatrix B_{NL}^i can be presented in terms of natural coordinates as

$$\begin{aligned} B_{NL}^{(i)(1,1)} &= \frac{1}{2} \frac{\zeta_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{\zeta_j(1+\eta_j\eta)}{2(ro^{(e)}-ri^{(e)})} U_j, \\ B_{NL}^{(i)(1,2)} &= \frac{1}{2} \frac{\zeta_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{\zeta_j(1+\eta_j\eta)}{2(ro^{(e)}-ri^{(e)})} V_j, \\ B_{NL}^{(i)(2,1)} &= \frac{1}{2} \frac{\eta_i(1+\zeta_i\zeta)}{\beta^{(e)}(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{\eta_j(1+\zeta_j\zeta)}{\beta^{(e)}(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} U_j \\ &\quad - \frac{\eta_i(1+\zeta_i\zeta)}{\beta^{(e)}(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{\eta_j(1+\zeta_j\zeta)(1+\eta_j\eta)}{2(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} V_j \\ &\quad + \frac{1}{2} \frac{(1+\zeta_i\zeta)(1+\eta_i\eta)}{2(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{(1+\zeta_j\zeta)(1+\eta_j\eta)}{2(\zeta(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} U_j, \end{aligned}$$

$$\begin{aligned}
 B_{NL}^{(i)(2,2)} &= \frac{1}{2} \frac{\eta_i(1+\xi_i\xi)}{\beta^e(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{\eta_j(1+\xi_i\xi)}{\beta^e(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} V_j \\
 &+ \frac{\eta_i(1+\xi_i\xi)}{\beta^e(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{(1+\xi_i\xi)(1+\eta_j\eta)}{2(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} U_j \\
 &+ \frac{1}{2} \frac{(1+\xi_i\xi)(1+\eta_i\eta)}{2(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} \sum_{j=1}^4 \frac{(1+\xi_i\xi)(1+\eta_j\eta)}{2(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} V_j, \\
 B_{NL}^{(i)(3,1)} &= \frac{\xi_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{\eta_j(1+\xi_i\xi)}{\beta^e(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} U_j \\
 &- \frac{\xi_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{(1+\xi_i\xi)(1+\eta_j\eta)}{2(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} V_j, \\
 B_{NL}^{(i)(3,2)} &= \frac{\xi_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{\eta_j(1+\xi_i\xi)}{\beta^e(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} V_j \\
 &+ \frac{\xi_i(1+\eta_i\eta)}{2(ro^{(e)}-ri^{(e)})} \sum_{j=1}^4 \frac{(1+\xi_i\xi)(1+\eta_j\eta)}{2(\xi(ro^{(e)}-ri^{(e)})+ro^{(e)}+ri^{(e)})} U_j.
 \end{aligned} \tag{40}$$

The components of \bar{B}_{NL} can be obtained with the same method applied for \bar{B}_{NL} . Hence, the virtual strain energy of Equation (33) takes the following form

$$\delta U = \int_A \delta \varepsilon^T \sigma dA = \delta \delta^{(e)T}, \int_A \bar{B}^T DB dA \delta^{(e)} \tag{41}$$

Employing the principle of virtual works, we reach the following equation

$$\int_{A^e} \bar{B}^T DB dA \delta^{(e)} = \left(\int_{l^e} N^T f dl \right)_{r=r_{out}} \tag{42}$$

where the force vector f based on the second Equation (33) is defined as $f = \{q \ 0\}^T$. Finally, the balance equation for each element reads as

$$K^{(e)} \delta^{(e)} = F^{(e)} \tag{43}$$

The stiffness matrix, $K^{(e)}$, and the force vector $F^{(e)}$, for each element, based on Equation (42), are

$$\begin{aligned}
 K^{(e)} &= \int_{A^e} \bar{B}^T DB dA, \\
 F^{(e)} &= \left(\int_{l^e} N^T f dl \right)_{r=r_{out}}
 \end{aligned} \tag{44}$$

Recalling the definitions $\bar{B} = B_L + \bar{B}_{NL}$ and $B = B_L + B_{NL}$, the stiffness matrix of an element may be defined as

$$\begin{aligned}
 K^{(e)} &= \int_{A^e} B_L^T DB_L dA + \int_{A^e} \bar{B}_{NL}^T DB_L dA \\
 &+ \int_{A^e} B_L^T DB_{NL} dA + \int_{A^e} \bar{B}_{NL}^T DB_{NL} dA. \\
 K_L^{(e)} &= \int_{A^e} B_L^T DB_L dA \\
 K_{NL1}^{(e)} &= \int_{A^e} \bar{B}_{NL}^T DB_L dA + \int_{A^e} B_L^T DB_{NL} dA \\
 K_{NL2}^{(e)} &= \int_{A^e} \bar{B}_{NL}^T DB_{NL} dA
 \end{aligned} \tag{45}$$

Hence, the final FE model of the circular ring is as follows

$$(K_L + K_{NL1} + K_{NL2}) \delta = F \tag{46}$$

Equation (46) is nonlinear and should be solved with iterative algorithms such as the Newton–Raphson scheme. Hence, the function $\Phi(\delta)$ is considered as

$$\Phi(\delta) = (K_L + K_{NL1} + K_{NL2})\delta = F \tag{47}$$

At the i th iteration, the tangent stiffness matrix becomes as

$$K_i^{tan} = \frac{d\Phi(\delta_i)}{d\delta_i} = K_L + 2K_{NL1} + 3K_{NL2} \tag{48}$$

Hence, Equation (46) gets the following form

$$K_i^{tan} \Delta\delta_{i+1} = -\Phi(\delta_i) \tag{49}$$

and δ_{i+1} is computed as

$$\delta_{i+1} = \Delta\delta_{i+1} + \delta_i. \tag{50}$$

4. Numerical Results and Discussion

In such a section as this, we first verify the accuracy of the proposed methodology, thus continuing with a systematic investigation of the post-buckling force-displacement curves of FG porous rings reinforced by GPLs for different input mechanical and geometrical parameters.

4.1. Validation of the Present Study

The post-buckling phase up to the collapse state of rings constructed of an isotropic material is here evaluated and compared with predictions by Kim and Chaudhuri [70]. To this end, it is enough to consider $\gamma_{GPL} = 0, e_0 = 0$. The mechanical and geometrical properties are considered exactly the same as in [70]. Such comparative evaluation among our results and the reference ones by Kim and Chaudhuri [70] is depicted in Figure 3 in terms of dimensionless load-displacement curves, where R_{in} refers to the inner radius of the ring and p_{cr} is the associated buckling load. Based on these plots, it is worth noting the perfect matching among all results, thus confirming the high accuracy of the proposed method against the available literature.

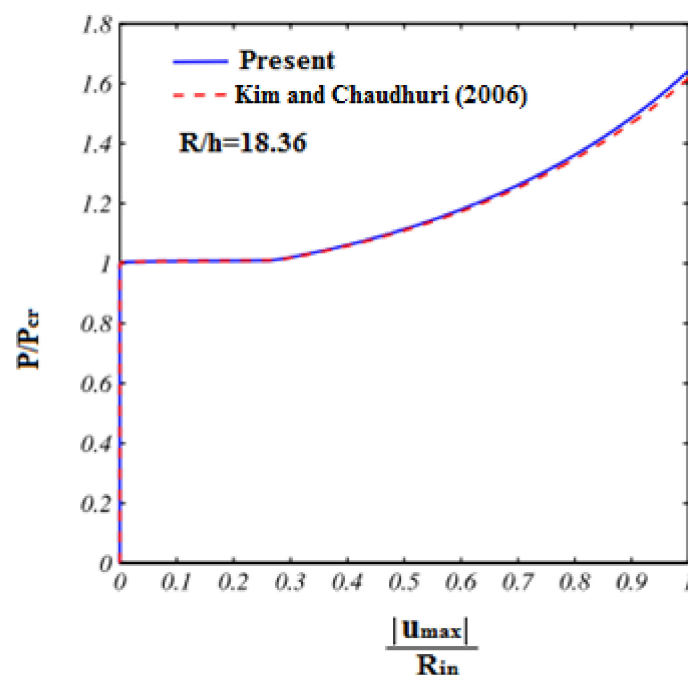


Figure 3. Post-buckling compressive state up to the collapse for an isotropic ring [70].

4.2. Post-Buckling Behavior of FG-GPL Porous Ring

In this section, we evaluate the post-buckling behavior of FG porous rings reinforced by GPLs for different porosity distributions and GPL patterns, porosity coefficients, and weight fractions of nanofillers. More specifically, the post-buckling behavior of the structure is here evaluated for two different points, A and B, as reported in Figure 1.

To this end, the geometry is defined by means of $r_{in} = 0.975$ m and $r_{out} = 1.025$ m, while assuming the following mechanical properties, i.e., $E_m = 130$ GPa, $\rho_m = 8960$ kg/m³, $\nu_m = 0.34$ and $E_{GPL} = 1.01$ TPa, $\rho_{GPL} = 1062.5$ kg/m³, $\nu_{GPL} = 0.186$, $w_{GPL} = 1.5$ μ m, $l_{GPL} = 2.5$ μ m, $t_{GPL} = 1.5$ nm for GPLs.

4.2.1. The Effect of GPLs Distributions and Weight Fractions

Figure 4 shows the influence of various GPL patterns on the post-buckling behavior of FG porous rings at point A. As can be seen, the equilibrium path shows a bifurcation point following a stable post-buckling path. Besides, the maximum buckling and post-buckling strength is related to a GPLX distribution, while the minimum values are associated to a GPL-V distribution. Also, the values of buckling load and post-buckling deformation are approximately the same for rings reinforced with GPL-V and GPL-O distributions, whose findings could be useful to scientists and engineers for many practical design purposes. Note that the maximum difference of critical buckling load for various GPL patterns is approximately 33%. Figure 5, also shows the effect of various weight fractions of nanofiller on the post-buckling response of FG porous ring reinforced by GPLs (always for point A). By increasing the weight fraction of nanofillers, the buckling and post-buckling strength of rings considerably increases (by approximately about 90%). On the other hand, by introducing about 1% of nanofiller, the stiffness of the ring increases significantly, together with its buckling strength.

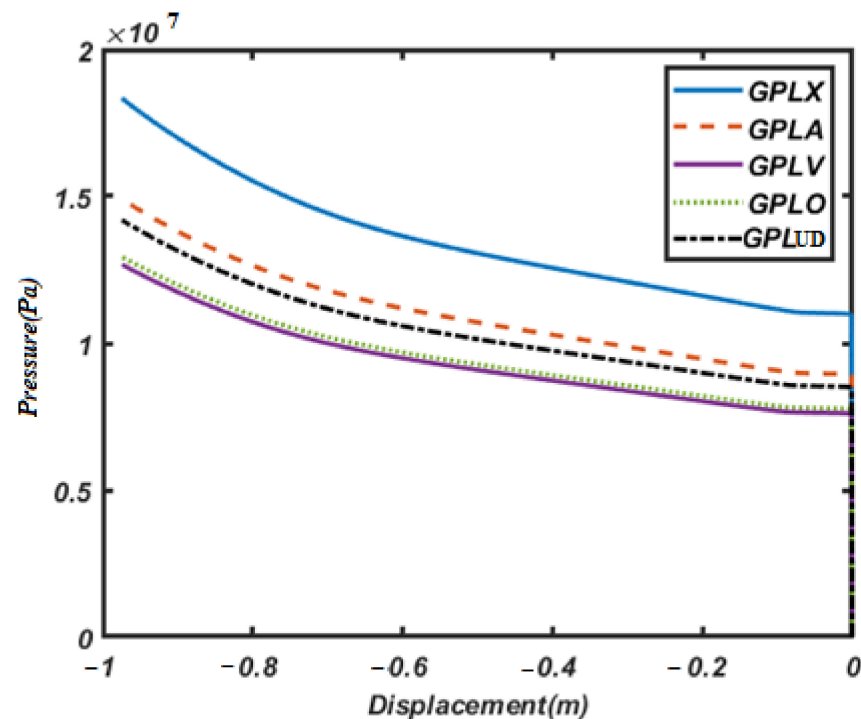


Figure 4. The effect of different GPL patterns on the post-buckling behavior of FG porous rings reinforced by GPLs (point A).

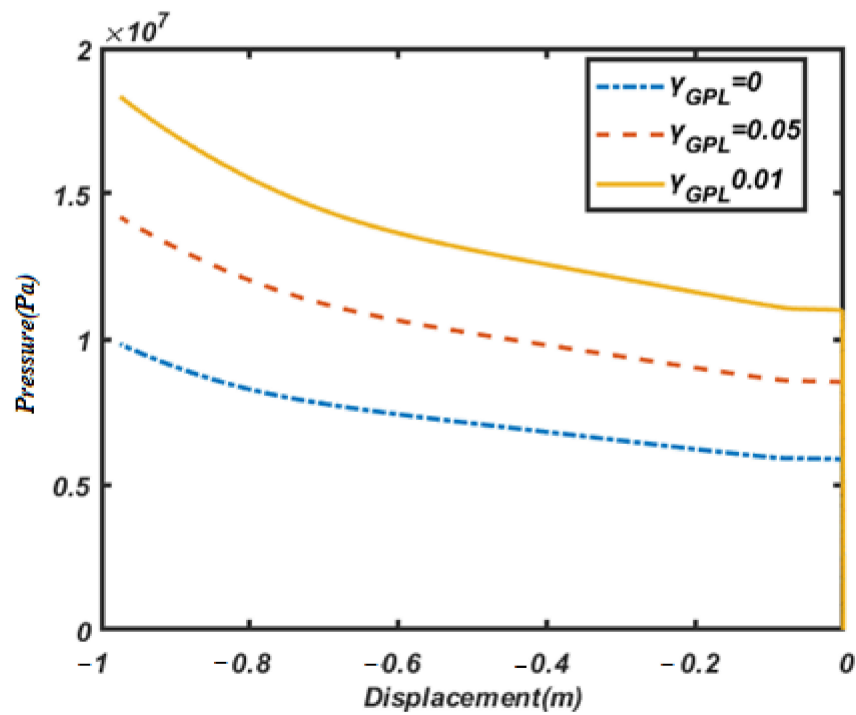


Figure 5. The effect of different weigh fraction of nanofiller on the post-buckling behavior of FG porous rings reinforced by GPLs (point A).

4.2.2. The Effect of Porosity Distribution and Coefficient of Porosity

Let us study now the effect of the porosity coefficient on the post-buckling behavior of FG porous rings reinforced by GPLs, whose results are depicted in Figure 6 for point A.

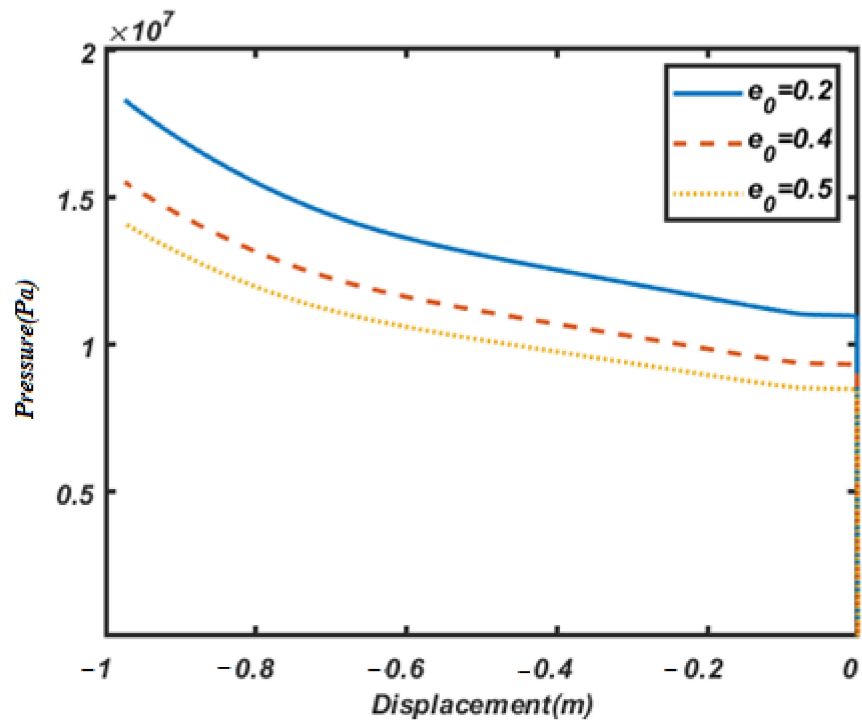


Figure 6. The effect of different porosity coefficients on the post-buckling behavior of FG porous rings reinforced by GPLs (point A).

By increasing the porosity coefficient, the buckling load and post-buckling strength of the ring decreases due to a decreased structural stiffness. By increasing the porosity coefficient from $e_0 = 0.2$ to $e_0 = 0.5$, the buckling load decreases by approximately of 28%. Figure 7 shows the influence of different porosity distributions on the post-buckling behavior of FG porous rings reinforced by graphene platelet (point A). The maximum buckling load and post-buckling strength is related to PD1 while the minimum values belong to PD2. It is worth observing, also, that the effect of porosity distribution on the buckling load and post-buckling strength of the ring is much more pronounced than that associated to the weight fraction of nanofillers or other parameters. On the other hand, the difference between the buckling load stemming from a PD1 and PD2 is approximately more than 100%. This means that the distribution of porosity plays an important role in the structural stiffness, which represents a key aspect for different engineering applications involving cellular and porous structures, primarily, the aerospace industry.

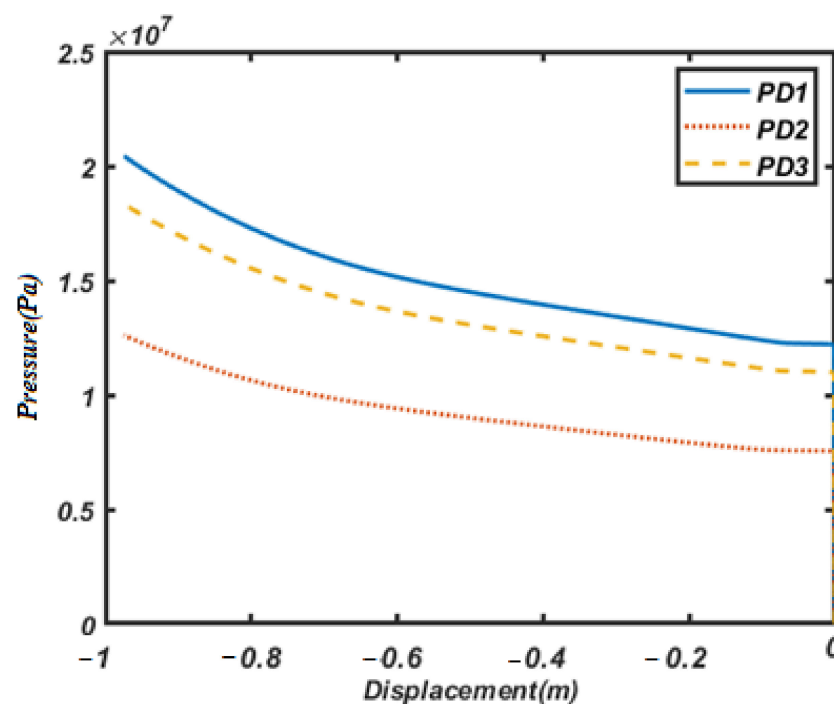


Figure 7. The effect of different porosity distributions on the post-buckling behavior of FG porous rings reinforced by GPLs (point A).

The same parametric investigation has been repeated for a different point, B, of the FG porous ring reinforced by graphene platelet, still concluding the same findings already highlighted for point A, as visible in Figures 8–11, despite the fact that the flattening phenomena are always observed when the structure starts collapsing. To study the post-buckling behavior of structures, most previous studies have referred to the post-buckling path based on a maximum displacement, although such maximum displacement may vary in any step with the point location. For a deep comprehension about the post-buckling behavior of structures, the post-buckling curves should be drawn for various points. Figure 12 finally represents the deformed configuration of the porous ring reinforced by graphene platelets in the post-buckling phase up to collapse for various loading configurations.

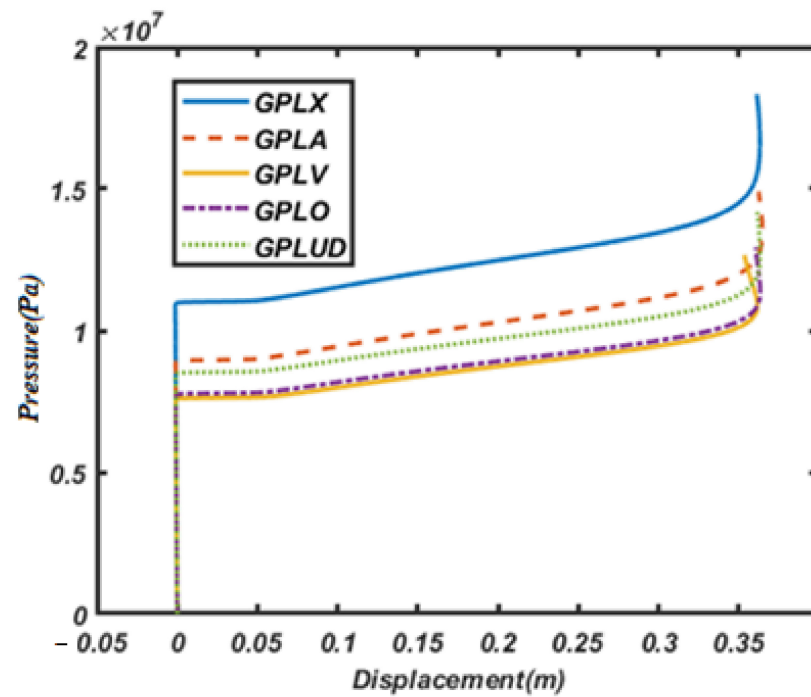


Figure 8. The effect of various GPL patterns on the post-buckling behavior of FG porous rings reinforced by GPLs (point B).

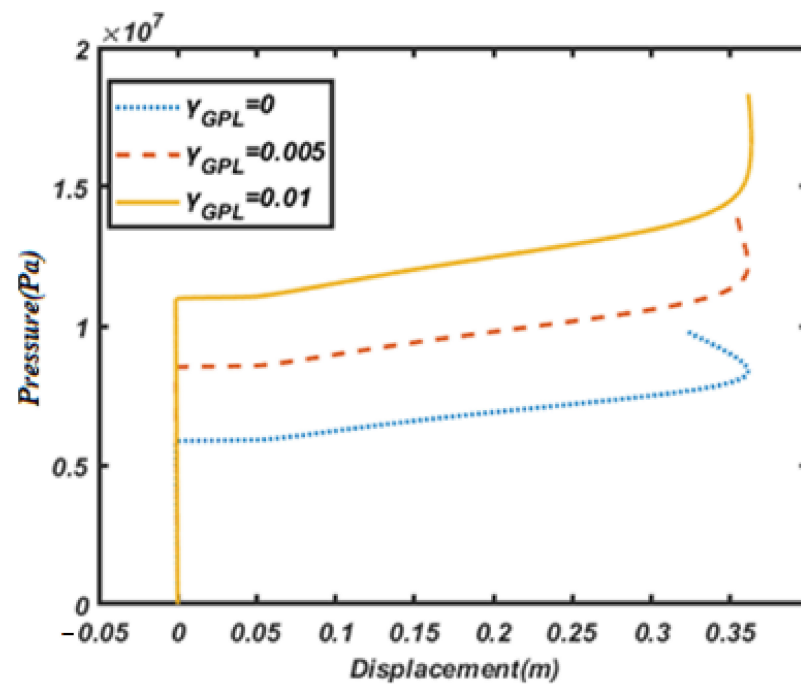


Figure 9. The effect of different weight fractions of nanofiller on the post-buckling behavior of FG porous rings reinforced by GPLs (point B).

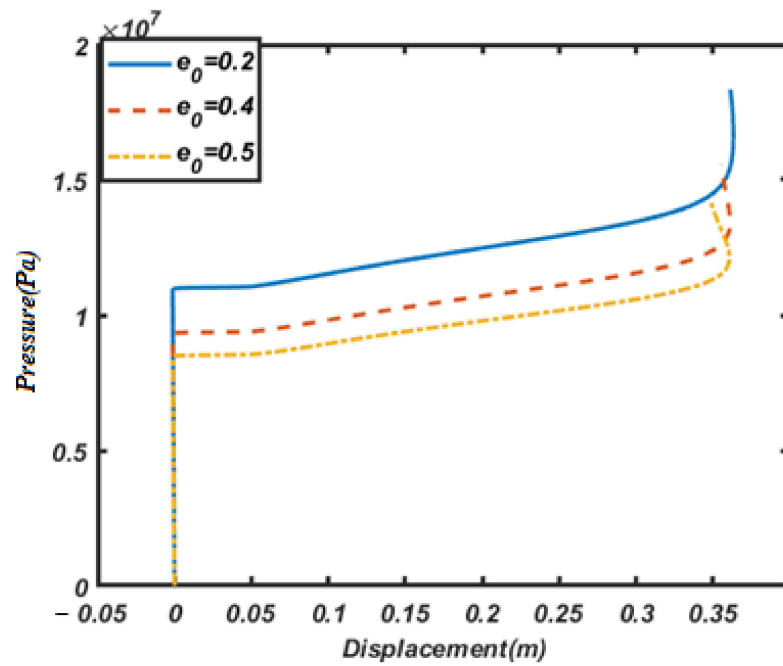


Figure 10. The effect of different porosity coefficients on the post-buckling behavior of FG porous rings reinforced by GPLs (point B).

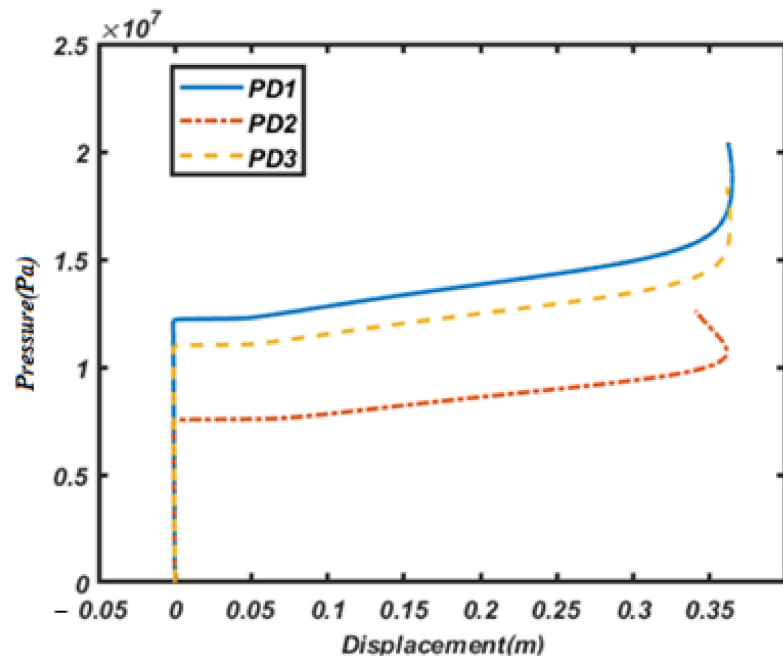


Figure 11. The effect of different porosity distributions on the post-buckling behavior of FG porous rings reinforced by GPLs (point B).

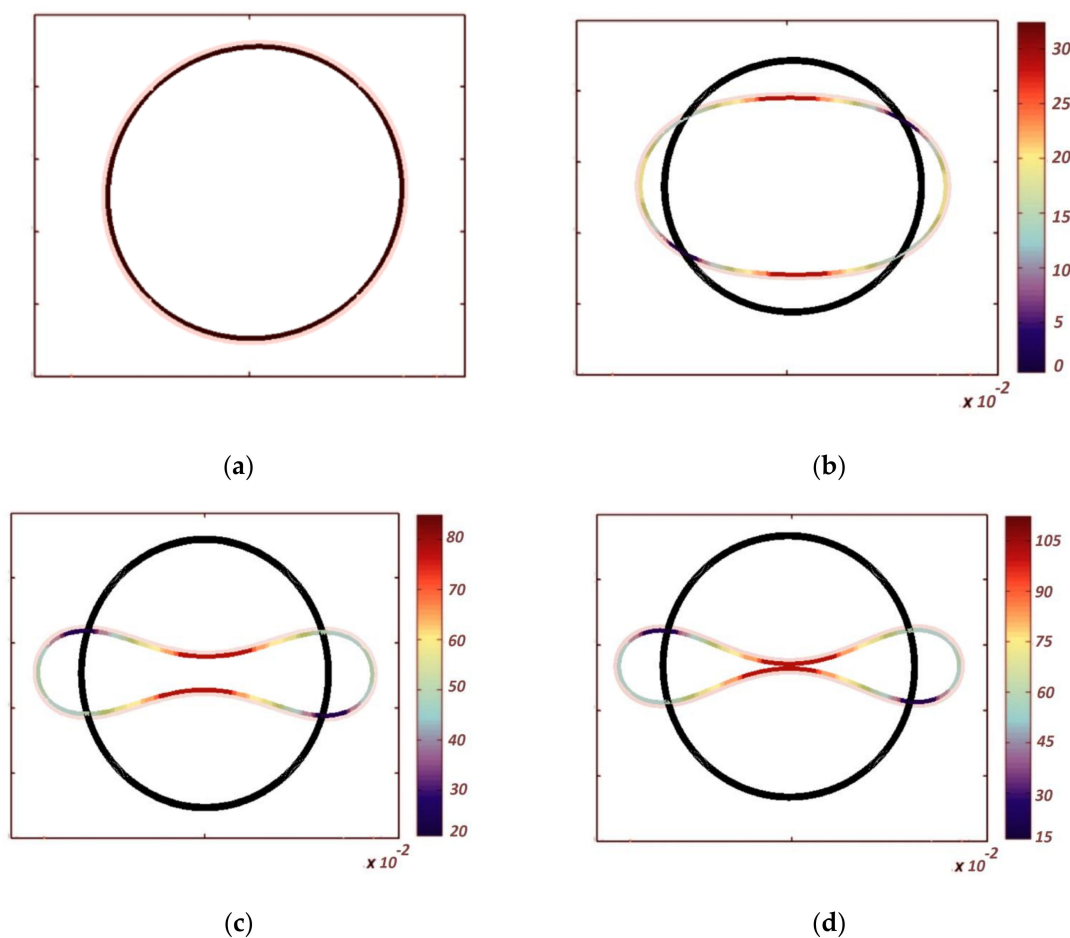


Figure 12. The deformed shape of an FG porous ring reinforced by GPLs under different loading conditions: 96 KPa (a), 9.15 MPa (b), 13.048 MPa (c), and 14.496 MPa (d).

5. Conclusions

Rings can be considered as the pipelines part in submarines structures, subjected to a hydrostatic uniform external pressure, and for which an accurate buckling analysis is required. Besides, FG porous reinforced GPLs can be considered as a novel material for many practical applications, primarily, for the aerospace industry.

In such a context, this article has focused on the post-buckling behavior up to the collapse state of rings constructed of FG porous materials reinforced with graphene platelet. A 2D plane stress elasticity theory has been proposed herein in conjunction with a finite element approach and iterative Newton–Raphson scheme to solve the governing equations of the ring. Due to the importance of an accurate prediction of the post-buckling path of rings, the 2D elasticity theory and the complete nonlinear Green strain field rather than simple shell theories and Von Kármán nonlinearities have been considered in this work. More specifically, a 2D elasticity theory has considered the thickness stretching and it has been revealed to predict accurately the post-buckling behavior of the selected structures. Based on the systematic sensitivity study of the buckling and post-buckling strength of FG-GPL porous rings, for different porosity coefficients, weight fractions of nanofiller, as well as different porosity distributions and GPL patterns, the following findings can be summarized:

- The maximum and minimum buckling loads and post-buckling strength of the ring belong to GPLX and GPL-O distributions. Besides, for GPL-V and GPL-O, the buckling load values and post-buckling paths are approximately the same;

- The maximum and minimum buckling loads and post-buckling strength of the ring belong to PD1 and PD2, respectively;
- The impact of porosity distribution on the buckling load and post-buckling path is more pronounced than the effect of weight fractions of nanofillers;
- By increasing the weight fraction of nanofiller about 1%, the buckling value and post-buckling strength considerably increase (by approximately 80%);
- By increasing the porosity coefficient from 0.2 to 0.5, the buckling load and post-buckling strength decreases (by approximately 28%);
- The behavior of the post-buckling path of the ring may vary from one point to another.

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